

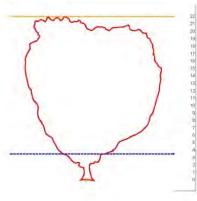






Wind Load Analysis analogous to DIN 1055-4

| Project | | | | Site | Tree Number | - 6 | 325 |
|---|---------|------------------------------|----------------------------|---------------------------------|-------------------------|-------------------------|-----|
| Project Name Project Number | Belle \ | /ue Par | K | Belle Vie Newport NP20 4F | w Park P Newport, UK | | |
| Test Date | 25.06. | 2010 | | | sea level | 30 | m |
| Tree Data | | | | Applied Ma | terial Properties | | |
| Tree Species Stem circumference | Quercu | s rubra 263 | cm | as for Source | Quercu S | s rubra tuttgart | |
| Stem Diameter in 1m height Bark Thickness (1m) Tree Height | 1 | 87 90 1,5 22 | om om om m | | | 20 7200 0,28 1 | MPa |
| | | | | | | | |



| Load Direction | NW / 303,5° |
|--------------------|-------------|
| Surface Area Anal | vsis |
| Crown Base | 3,5 m |
| Effective Height | 14,6 m |
| Total Surface Area | 270 m² |
| Crown Eccentricity | 0.69 m |

 Applied Structural Parameters

 Drag Factor
 0,25

 Eigenfrequency
 0,43
 Hz

 Dampling Ratio
 0,54

 Form Factor for Dead Weight
 0,8

Applied Site Parameters
Windzone
Speed of Applied
Design Wind Speed
Air Density
Roughness Catagory
Landscape
Exponent for Wind Profile
Proximity Factor for Effects
in Near Ground Wind Flow
Factor for Crown Exposure
0,7

| Results | | | | |
|----------------------|------|-----|----------------------------------|--------|
| Wind Load Analysis | | | Tree Static Analysis | |
| Mean Wind Pressure | 18,9 | kN | Dead Weight Tree | 10,1 |
| Gust Reaction Factor | 2,11 | | Critical Degree of Hollowness | 82 |
| Load Centre | 13,3 | m | Critical Residual Wall Thickness | ss 8 |
| Torsion Moment | 27 | kNm | Assuming an Uncompromised | Residu |
| Design Wind Load | 527 | kNm | Basic Safety Factor | 2,2 |
| | | | | |

General Comments

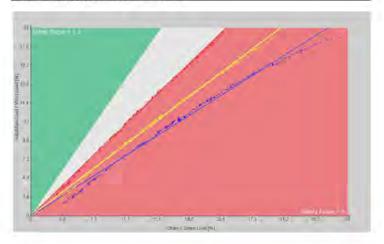
Crown Outline



Calculated Fracture Stability according to Pull Test

| Belle Vue Park Quercus rubra | Tree Number Date | 625 25.06.2010 |
|---------------------------------|-----------------------------------|-----------------------|
| | | |
| 8,5 m 17 ° | Measurement No. Load Direction | NW / 303.5° |
| | Quercus rubra | 8,5 m Measurement No. |

Graphic Display (test data and best linear fit)

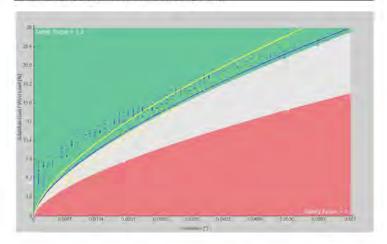


| Elastometer Measurement | in | 90 | 91 | 92 | |
|-----------------------------|--------------------|------------|---------------|-------------|--|
| Measurement Height | m | 0,6 | 0,9 | 0,85 | |
| Position | | TEN | COM | TEN | |
| Stem Diameter 1 | cm | 107 | 85 | 93 | |
| Stem Diameter 2 | cm | 112 | 91 | 97 | |
| Bark Thickness (1m) | cm | 1,5 | 1,5 | 1,5 | |
| Breaking Stability (derived | from t | he gradien | t of the best | linear fit) | |
| Safety Factor | | 0,66 | 0,78 | 1 | |
| Control Value | | | | | |
| Coefficient of Determinal | ion R ² | 0,9915 | 0,9976 | 0,9985 | |
| Residual Stiffness | % | 15 | 34 | 33,8 | |
| Degree of Hollowness | % | 94,7 | 87,1 | 87,2 | |
| Compression originating | from | | | | |
| Dead Weight | % | 5,3 | 3,5 | 3 | |
| Substitute Load | % | 22,8 | 23 | 23,7 | |
| | | | | | |

Calculated Tipping Stability according to Pull Test

| Project | Belle Vue Park | Tree Number | 625 |
|---|----------------|-----------------|------------|
| Tree Species | Quercus rubra | Date | 25.06.2010 |
| | | | |
| Setup Pulling Test | | | |
| Setup Pulling Test Height of the Stern Anchor | 8,5 m | Measurement No. | - 1 |

Graphic Display (test data and best fit to tipping curve)



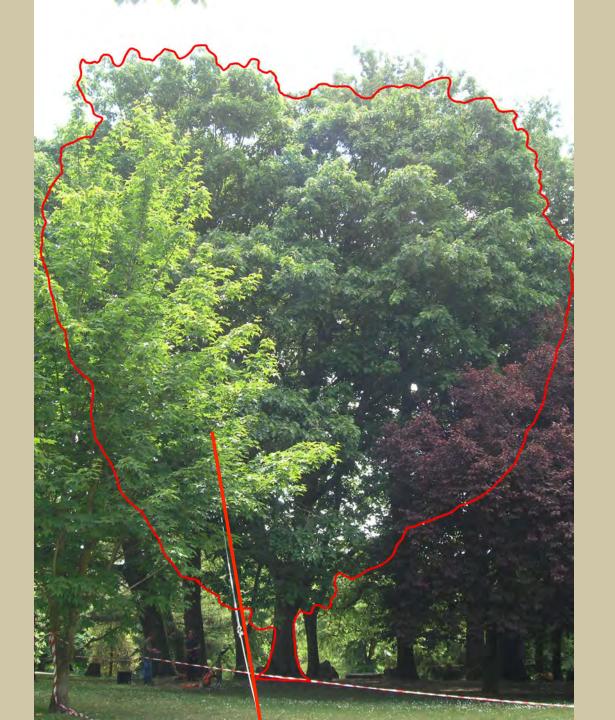
| Inclinometer Measureme | ent | 80 | 81 | |
|---|-------------|----------------|----------------|--|
| Position | | SW | NNW | |
| Tipping Stability (based | on Gen | eralized Tipp | ing Curve) | |
| Safety Factor | | 1,54 | 1,7 | |
| Control Value | in | | | |
| Standard Deviation | % | 2,5 | 1,63 | |
| Substitute Load Load Direction at Inclin | % ometer | 25,6 x-Axis | 25,6 x-Axis | |
| General for Pull Test | | | | |
| Consultant Witness / Assistant | | Paul Muir | | |
| Measurement Commer | nts | | | |
| | | | | |

○ ArboSafe

Treework Environmental Practice, Bristol OArboSafe Treework Environmental Practice, Bristol







Implications



Strain data (prior to crack formation) demonstrate very low safety margins (depending on appropriate WLA) and a reaction equivalent to an extremely thin-walled hollow shell

Longitudinal crack likely to have been the result of unusually high level of torsion applied

Would a pure bending moment have initiated this longitudinal crack?

Would a pure bending moment have resulted in a similar reaction in terms of strain?

Would the test have predicted failure if a 100 micron threshold had been followed?

